21 .I Introduction

Thin shell theory is satisfactory when the thickness of the shell divided by its radius is less than 1/30. When the thickness: radius ratio of the shell is greater than this, errors start to occur and thick shell theory should be used. Thick shells appear in the form of gun barrels, nuclear reactor pressure vessels, and deep diving submersibles.

21.2 Derivation of the hoop and radial stress equations for a thickwalled circular cylinder

The following convention will be used, where all the stresses and strains are assumed to be tensile and positive. At any radius, *r*

- $\sigma_{\rm g}$ = hoop stress
- *or* = radial stress
- **o,** = longitudinal stress
- ε_{θ} = hoop strain
- ϵ _r = radial strain

Figure 21.1 Thick cylinder.

E, = **longitudinzl strain (assumed to be constant)**

w = **radial deflection**

From Figure 21.2, it can be seen that **at any radius** *r,*

$$
\varepsilon_{\theta} = \frac{2\pi(r+w) - 2\pi r}{2\pi r}
$$

or

 ε_{θ} = *w/r* (21.1)

Similarly,

$$
\varepsilon_r = \frac{\delta w}{\delta r} = \frac{dw}{dr} \tag{21.2}
$$

Figure 21.2. Deformation at any radius *r.*

From the standard stress-strain relationships,

$$
E\varepsilon_z = \sigma_z - \nu \sigma_{\theta} - \nu \sigma_r = \text{a constant}
$$

$$
E\varepsilon_{\theta} = \frac{Ew}{r} = \sigma_{\theta} - \nu \sigma_z - \nu \sigma_r
$$
 (21.3)

$$
E\varepsilon_r = E\frac{dw}{dr} = \sigma_r - \nu\sigma_\theta - \nu\sigma_z \tag{21.4}
$$

Multiplying equation (21.3) by r ,

$$
Ew = \sigma_{\theta} \times r - \nu \sigma_{z} \times r - \nu \sigma_{r} \times r
$$
 (21.5)

and differentiating equation (21.5) with respect to r , we get

$$
E\frac{dw}{dr} = \sigma_{\theta} - v\sigma_{z} - v\sigma_{r} + r\left(\frac{d\sigma_{\theta}}{dr} - v\frac{d\sigma_{z}}{dr} - v\frac{d\sigma_{r}}{dr}\right)
$$
(21.6)

Subtracting equation (21.4) from equation (21.6),
\n
$$
(\sigma_{\theta} - \sigma_r)(1 + v) + r \frac{d\sigma_{\theta}}{dr} - v r \frac{d\sigma_r}{dr} - v r \frac{d\sigma_r}{dr} = 0
$$
\n(21.7)

As ε _z is constant

$$
\sigma_z - v \sigma_\theta - v \sigma_r = \text{constant} \tag{21.8}
$$

Differentiating equation (21.8) with respect to r ,

$$
\frac{d\sigma_z}{dr} - v \frac{d\sigma_{\theta}}{dr} - v \frac{d\sigma_r}{dr} = 0
$$

or

$$
\frac{d\sigma_z}{dr} = v \left(\frac{d\sigma_{\theta}}{dr} + \frac{d\sigma_r}{dr} \right)
$$
 (21.9)

Substituting equation (21.9) into equation (21.7),

$$
(\sigma_{\theta} - \sigma_r)(1 + v) + r(1 - v^2)\frac{d\sigma_{\theta}}{dr} - vr(1 + v)\frac{d\sigma_r}{dr} = 0
$$
\n(21.10)

and dividing equation (21.10) by $(1 + v)$, we get

$$
\sigma_{\theta} - \sigma_r + r (1 + v) \frac{d\sigma_{\theta}}{dr} - vr \frac{d\sigma_r}{dr} = 0
$$
 (21.11)

Considering now the radial equilibrium of the shell element, shown in Figure 21.3,

Figure 21.3 Shell element.

$$
2\sigma_{\theta} \quad \delta_r \sin\left(\frac{\delta\theta}{2}\right) + \sigma_r \quad r \quad \delta_{\theta} - (\sigma_r + \delta\sigma_r)(r + \delta r)\delta\theta = 0 \tag{21.12}
$$

Neglecting higher order terms in the above, we get

$$
\sigma_{\theta} - \sigma_r - r \frac{d\sigma_r}{dr} = 0 \tag{21.13}
$$

Subtracting equation (21.11) from equation (21.12)

$$
\frac{d\sigma_{\theta}}{dr} + \frac{d\sigma_{r}}{dr} = 0 \tag{21.14}
$$

$$
\therefore \sigma_{\theta} + \sigma_{r} = \text{constant} = 2A \tag{21.15}
$$

Subtracting equation (21.13) from equation (21.15),

$$
2\sigma_r + r\frac{d\sigma_r}{dr} = 2A
$$

or

$$
\frac{1}{r} \frac{d(\sigma_r r^2)}{dr} = 2A
$$

$$
\frac{d(\sigma_r r^2)}{dr} = 2Ar
$$

Integrating the above,

$$
\sigma_r \quad r^2 = Ar^2 - B
$$
\n
$$
\sigma_r = A - \frac{B}{r^2}
$$
\n(21.16)

From equation (21.15) ,

$$
\sigma_{\theta} = A + \frac{B}{r^2} \tag{21.17}
$$

21.3 Lame line

If equations (21.16) and (21.17) are plotted with respect to a horizontal axis, where $1/r^2$ is the horizontal axis, the two equations appear as a single straight line, where σ , lies to the left and σ ₀ to the right, as shown by Figure 21.4. For the case shown in Figure 21.4, σ , is compressive and $\sigma_{\rm A}$ tensile, where

 $\sigma_{\theta1}$ = internal hoop stress, which can be seen to be the maximum stress

 $\sigma_{\theta2}$ = external hoop stress

Figure 21.4 Lame line for **the case** of **internal pressure.**

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To calculate σ_{θ_1} and σ_{θ_2} , equate similar triangles in Figure 21.4,

$$
\frac{\sigma_{\theta 1}}{\left(\frac{1}{R_1^2} + \frac{1}{R_2^2}\right)} = \frac{P}{\left(\frac{1}{R_1^2} - \frac{1}{R_2^2}\right)}
$$

or

$$
\sigma_{\theta 1} = \frac{P\left(\frac{1}{R_1^2} + \frac{1}{R_2^2}\right)}{\left(\frac{1}{R_1^2} - \frac{1}{R_2^2}\right)} \times \frac{R_1^2 R_2^2}{R_1^2 R_2^2}
$$
\n
$$
\sigma_{\theta 1} = \frac{P\left(R_1^2 + R_2^2\right)}{\left(R_2^2 - R_1^2\right)}
$$
\n(21.18)

Similarly, from Figure 21.4

$$
\frac{\sigma_{\theta 2}}{\left(\frac{1}{R_2^2} + \frac{1}{R_2^2}\right)} = \frac{P}{\left(\frac{1}{R_1^2} - \frac{1}{R_2^2}\right)}
$$

or,

$$
\sigma_{\theta 2} = \frac{P\left(\frac{1}{R_2^2}\right) \times 2}{\left(\frac{1}{R_1^2} - \frac{1}{R_2^2}\right)} \times \frac{R_1^2 R_2^2}{R_1^2 R_2^2}
$$
\n(21.19)\n
$$
\sigma_{\theta 2} = \frac{2PR_1^2}{\left(R_2^2 - R_1^2\right)}
$$

Problem 21.1 A thick-walled circular cylinder of internal diameter 0.2 m is subjected to an internal pressure of 100 MPa. If the maximum permissible stress in the cylinder is limited to 150 MPa, determine the maximum possible external diameter *d,.*

Solution

Figure 21.5 Lamé line for thick cylinder.

or

$$
\frac{\left(\frac{1}{0.2^2} + \frac{1}{d_2^2}\right)}{\left(\frac{1}{0.2^2} - \frac{1}{d_2^2}\right)} \times \left(\frac{0.2^2}{0.2^2} \frac{d_2^2}{d_2^2}\right) = 1.5
$$

$$
\left(\frac{d_2^2 + 0.2^2}{d_2^2 - 0.2^2}\right) = 1.5
$$

or
$$
d_2^2 + 0.2^2 = 1.5 \left(d_2^2 - 0.2^2 \right)
$$

or
$$
0.2^2(1+1.5) = d_2^2(1.5-1)
$$

$$
d_2^2 = 0.2 \,\mathrm{m}^2
$$

$$
d_2 = 0.447 \,\mathrm{m}
$$

Problem 21.2 If the cylinder in the previous problem were subjected to an external pressure **of** 100 MPa and an internal pressure of zero, what would be the maximum magnitude of stress.

Solution

Now $\frac{1}{d^2}$ = 25 and $\frac{1}{d^2}$ = 5, d_1^2 d_2^2

hence the Lamé line would take the form of Figure 21.6.

Figure 21.6 Lamé line for external pressure case.

By equating similar triangles,

$$
\frac{-100}{(25-5)} = \frac{\sigma_{\theta_i}}{25 + 25}
$$

where $\sigma_{\theta i}$ is the internal stress which has the maximum magnitude

$$
\therefore \sigma_{\theta i} = \frac{-50 \times 100}{20} = -250 \text{ MPa}
$$

Problem 21.3 A steel disc of external diameter 0.2 m and internal diameter 0.1 m is shrunk onto a solid steel shaft of external diameter 0.1 m, where all the dimensions are nominal. If the interference fit, based on diameters, between the shaft and the disc at the common surface is 0.2 mm, determine the maximum stress. For steel, $E = 2 \times 10^{11}$ N/m², $v = 0.3$

Solution

Consider the steel disc. In this case the radial stress on the internal surfaces is the unknown $P_{\rm c}$. Hence, the Lamé line will take the form shown in Figure 21.7.

Figure 21.7 Lame line for **steel ring.**

Let,

 $\sigma_{\theta d}$ = hoop stress (maximum stress) on the internal surface of the disc

 σ_{rld} = radial stress on the internal surface of the disc

Equating similar triangles, in Figure 21.7

$$
\sigma_{r1d} = \text{radial stress on the internal surface}
$$
\n
$$
\text{similar triangles, in Figure 21.7}
$$
\n
$$
\frac{P_c}{(100 - 25)} = \frac{\sigma_{\text{old}}}{100 + 25}
$$
\n
$$
\therefore \sigma_{\theta1d} = \frac{125 \ P_c}{75} = 1.667 \ P_c
$$

Consider now the solid shaft. In this case, the internal diameter of the shaft is zero and as $1/0^2$ \rightarrow **m,** the Lam6 line must be horizontal or the shaft's hoop stress will be **infinity,** which is impossible; see Figure **21.8.**

Figure 21.8 Lam6 line for a solid shaft.

Let

 P_{c} = external pressure on the shaft

$$
\sigma_r = \sigma_\theta = -P_c \text{ (everywhere)}
$$
 (21.20)

Let,

 w_d = increase in the radius of the disc at its inner surface

 w_s = increase in the radius of the shaft at its outer surface

Now, applying the expression

$$
E\varepsilon_{\theta} = \frac{w}{r} = \sigma_{\theta} - v\sigma_{r} - v\sigma_{x}
$$

to the inner surface *of* the disc

$$
\frac{EW_d}{5 \times 10^{-2}} = \sigma_{\theta \text{ld}} - \nu \sigma_{\text{rld}}
$$

but,

 σ_{reld} = $-P_c$

therefore

$$
\frac{2 \times 10^{11} \times w_d}{5 \times 10^{-2}} = 1.667 P_c + 0.3 P_c
$$

$$
w_d = 4.918 \times 10^{-13} P_c
$$
 (21.21)

Similarly, for the **shaft**

$$
\frac{Ew_s}{5\times 10^{-2}} = \sigma_{\theta s} - v\sigma_{rs}
$$

but

$$
\sigma_{\theta s} = \sigma_{rs} = P_c
$$

$$
2 \times 10^{11} w_s = -P_c (1 - v)
$$

$$
w_s = -1.75 \times 10^{-13} P_c \tag{21.22}
$$

but
$$
w_d - w_s = 2 \times 10^{-3}/2
$$

$$
(4.918 \times 10^{-13} + 1.75 \times 10^{-13}) P_c = 1 \times 10^{-4}
$$

$$
\therefore P_c = 150 \text{ MPa}
$$

Maximum stress is

 σ_{std} = 1.667 P_c = 250 MPa

21.4 Compound tubes

A compound tube is usually made from two cylinders of different materials where one is shrunk onto the other.

Problem 21.4 A circular steel cylinder of external diameter 0.2 m and internal diameter 0.1 m is shrunk onto a circular aluminium alloy cylinder of external diameter 0.1 m and internal diameter 0.05 m, where the dimensions are nominal.

Determine the radial pressure at the common surface due to shrinkage alone, *so* that when there is an internal pressure of 300 MPa, the maximum hoop stress in the inner cylinders is 150 Mpa. Sketch the hoop stress distributions. For steel, $E_s = 2 \times 10^{11} \text{ N/m}^2$, $v_s = 0.3$ For aluminium alloy, $E_a = 6.7 \times 10^{10} \text{ N/m}^2$, $v_a = 0.32$

Solution

Consider first the stress due to shrinkage alone, as shown in Figures 21.9 and 21.10.

Figure 21.9 Lamé line for aluminium alloy tube.

Figure 21.10 Lamé line for steel tube, due to shrinkage with respect to e.

Equating similar triangles in Figure 21.9.

$$
\frac{\sigma_{\theta,5a}^s}{400 + 400} = \frac{-P_c^s}{400 - 100}
$$
\n(21.23)\n
$$
\sigma_{\theta,5a}^s = -2.667 \ P_c^s
$$

Similarly, from figure 21.9,

$$
\frac{\sigma_{\theta, 1a}^s}{400 + 100} = \frac{-P_c^s}{400 - 100}
$$
\n(21.24)\n
$$
\sigma_{\theta, 1a}^s = -1.667 P_c^s
$$

Equating similar triangles in Figure 2 1.10.

$$
\frac{\sigma_{\theta, 1s}^s}{100 + 25} = \frac{P_c^s}{100 - 25}
$$
\n
$$
\sigma_{\theta, 1s}^s = 1.667 P_c^s
$$
\n(21.25)

Consider the stresses due to **pressure alone**

P, = **internal pressure**

P,' = **pressure at the common surface due to pressure alone** The Lamé lines will be as shown in Figures 21.11 and 21.12.

Figure 21.11 Lamé line in aluminium alloy, due to pressure alone.

Figure 21.12 Lamé line for steel, due to pressure alone.

Equating similar triangles in Figure 21.11.

$$
\frac{P - P_c^P}{400 - 100} = \frac{\sigma_{\theta,1a}^P + P}{400 + 100}
$$

or
$$
\frac{300 - P_c^P}{300} = \frac{\sigma_{\theta,1a}^P + 300}{500}
$$
 (21.26)

or $\sigma_{0.1a}^P = 200 - 1.667 P_c^P$

Similarly, from Figure 21.11,

$$
\frac{P - P_c^P}{300} = \frac{\sigma_{\theta,5a}^P + P}{800}
$$

$$
\frac{300 - P_c^P}{300} = \frac{\sigma_{\theta,5a}^P + 300}{800}
$$
 (21.27)

or
$$
\sigma_{\theta,5a}^P = \frac{8}{3} (300 - P_c^P) - 300
$$

$$
\sigma_{\theta,5a}^P = 500 - 2.667 P_c^P
$$

Similarly, from Figure 21.12,

$$
\frac{\sigma_{\theta,1s}^P}{100+25} = \frac{P_c^P}{100-25}
$$
\n
$$
\sigma_{\theta,1s}^P = 1.667 P_c^P
$$
\n(21.28)

Owing *to* **pressure alone, there is** *no interferencefit,* **so that**

$$
w_a^P = w_s^P
$$

Now

$$
\frac{E_s w_s^P}{0.05} = \sigma_{\theta, 1s}^P + v_s P_c^P
$$

 \overline{or}

$$
w_s^P = \frac{0.05}{2 \times 10^{11}} \left(1.667 \ P_c^P + 0.3 \ P_c^P \right)
$$

 $w_a^P = \frac{0.05}{6.7 \times 10^{10}} \left(\sigma_{\theta, 1a}^P + 0.32 P_c^P \right)$

or
$$
w_s = 4.917 \times 10^{-13} P_c^P
$$
 (21.29)

Similarly

$$
\frac{E_a w_a^P}{0.05} = \sigma_{\theta,1a}^P + v_a P_c^P
$$

or

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$$
= \frac{0.05}{6.7 \times 10^{10}} \left(200 - 1.667 P_c^P + 0.32 P_c^P\right)
$$
\n
$$
w_a^P = 1.493 \times 10^{-10} - 1.0 \times 10^{-12} P_c^P
$$
\n(21.30)

Equating (21.29) **and** (21.30)

$$
4.917 \times 10^{-13} P_c^P = 1.493 \times 10^{-10} - 1.0 \times 10^{-12}
$$
\n
$$
\therefore P_c^P = 100 MPa
$$
\n(21.31)

Substituting equation (21.31) into equations (21.26) and (21.27)

$$
\sigma_{\theta,5a} = 500 - 2.667 \times 100 = 233.3 \text{ MPa}
$$
 (21.32)

$$
\sigma_{\theta, 1a}^P = 200 - 1.667 \times 100 = 33.3 \text{ MPa}
$$
 (21.33)

Now the maximum **hoop** stress in the inner tube lies either on its internal surface or its external surface, **so** that either

$$
\sigma_{\theta, 1a}^P + \sigma_{\theta, 1a}^s = 150 \tag{21.34}
$$

or

$$
\sigma_{\theta,5a}^P + \sigma_{\theta,5a}^s = 150 \tag{21.35}
$$

Substituting equations (21.32) and (21.24) into equation (21.34), we get

33.3 - 1.667
$$
P_c^s
$$
 = 150
or P_c^s = -70 MPa

Substituting equations (21.33) and (21.23) into equation (21.39, we get

233.3 - 2.667
$$
P_c^s = 150
$$

 $\therefore P_c^s = 31.2 \text{ MPa}$

i.e. $P_c^s = 31.2 \text{ MPa}$, as P_c^s cannot be negative!

$$
P_c = P_c^s + P_c^P = 31.2 + 100 = 131.2 \text{ MPa}
$$
 (21.36)

$$
\frac{\sigma_{\theta,2s}}{25 + 25} = \frac{P_c^s + P_c^{\prime\prime}}{100 - 25}
$$
\n
$$
\sigma_{\theta,1s} = 87.5 \text{ MPa}
$$
\n
$$
\sigma_{\theta,1s} = 1.667 \left(P_c^s + P_c^{\prime\prime} \right) = 218.7 \text{ MPa}
$$
\n
$$
\sigma_{\theta,1a} = 200 - 1.667 \left(P_c^s + P_c^{\prime\prime} \right) = -18.7 \text{ MPa}
$$
\n
$$
\sigma_{\theta,5a} = 500 - 2.667 \left(P_c^s + P_c^{\prime\prime} \right) = 150 \text{ MPa}
$$

Figure 21.13 Hoop stress distribution.

21.5 Plastic deformation of thick tubes

The following assumptions will be made in this theory:

- 1. Yielding will take place according to the maximum shear stress theory, (Tresca).
- 2. The material of construction will behave in an ideally elastic-plastic manner.
- 3. The longitudinal stress will be the 'minimax' stress in the three-dimensional system of stress.

For this case, the equilibrium considerations of equation (21.13) apply, so that

$$
\sigma_{\theta} - \sigma_r - r \frac{d\sigma_r}{dr} = 0 \tag{21.37}
$$

Now, according to the maximum shear stress criterion of yield,

$$
\sigma_{\theta} - \sigma_{r} = \sigma_{yp}
$$

\n
$$
\sigma_{\theta} = \sigma_{yp} + \sigma_{r}
$$
\n(21.38)

Substituting equation (21.38) into equation (21.37),

$$
\sigma_{yp} + \sigma_r - \sigma_r - r \frac{d\sigma_r}{dr} = 0
$$

$$
d\sigma_r = \sigma_{yp} \frac{dr}{r}
$$
 (21.39)

$$
\sigma_r = \sigma_{yp} \ln r + C
$$

For the case of the partially plastic cylinder shown in Figure 21.14,

at $r = R_2$, $\sigma_r = -P_2$

Substituting this boundary condition into equation (21.39), we get

$$
-P_2 = \sigma_{vp} \ln R_2 + C
$$

therefore

$$
C = -\sigma_{\text{yp}} \ln R_2 - P_2
$$

and,

$$
\sigma_r = \sigma_{yp} \ln \left(\frac{r}{R_2} \right) - P_2 \tag{21.40}
$$

Similarly, from equation (21.38),

$$
\sigma_{\theta} = \sigma_{\mu} \left\{ 1 + \ln \left(\frac{r}{R_2} \right) \right\} - P_2 \tag{21.41}
$$

where,

- R_1 = internal radius
- R_2 = outer radius of plastic section of cylinder
- $R₃$ = external radius
- *P,* = internal pressure
- *P,* = external pressure

Figure 21.14 Partially **plastic** cylinder.

The vessel can be assumed to behave as a compound cylinder, with the internal portion behaving plastically, and the external portion elastically. The Lamé line for the elastic portion of the cylinder is shown in Figure 21.15.

Figure 21.15 Lamé line for elastic zone.

In Figure 21.15,

$$
\hat{\sigma}_{\theta e} = \text{elastic hoop stress at } r = R_2
$$

so that according to the maximum shear stress criterion of yield on this radius,

$$
\sigma_{yp} = \hat{\sigma}_{\theta e} + P_2 \tag{21.42}
$$

From Figure 21.15

$$
\frac{P_2}{\left(\frac{1}{R_2^2} - \frac{1}{R_3^2}\right)} = \frac{\hat{\sigma}_{\theta e}}{\left(\frac{1}{R_2^2} + \frac{1}{R_3^2}\right)}
$$

therefore

$$
\hat{\sigma}_{\theta e} = \frac{P_2(R_3^2 + R_2^2)}{(R_3^2 - R_2^2)}
$$
 (21.43)

Substituting equation (21.43) into equation (21.42),

$$
P_2 = \sigma_{yp} (R_3^2 - R_2^2) / (2R_3^2) \tag{21.44}
$$

Consider now the portion of the cylinder that is plastic. Substituting equation (2 1.44) into equation (21.41) , the stress distributions in the plastic zone are given by:

$$
\sigma_r = -\sigma_{yp} \left\{ \ln \left(\frac{R_2}{r} \right) + \frac{\left(R_3^2 - R_2^2 \right)}{2R_3^2} \right\}
$$
 (21.45)

$$
\sigma_{\theta} = \sigma_{\gamma p} \left\{ \frac{\left(R_3^2 + R_2^2\right)}{2R_3^2} - \ln \left(\frac{R_2}{r}\right) \right\} \tag{21.46}
$$

To find the *pressure* to *just cause yield,* put

$$
\sigma_r = -P_1 \quad \text{when} \quad r = R_1
$$

where P_i is the internal pressure that causes the onset of yield. Therefore,

$$
P_1 = \sigma_{yp} \left\{ \ln \left(\frac{R_2}{R_1} \right) + \left(\frac{R_3^2 - R_2^2}{2R_3^2} \right) \right\}
$$
 (21.47)

but, if yield is **only** on the inside surface,

$$
R_1 = R_2
$$

in (21.61), so that,

$$
P_1 = \sigma_{yp} \left\{ \left| R_3^2 - R_1^2 \right| / \left(2R_3^2 \right) \right\} \tag{21.48}
$$

To determine the *plastic collapse pressure* P_p , put $R_2 = R_3$ in equation (21.47), to give

$$
P_P = \sigma_{yp} \ln \left(\frac{R_3}{R_1} \right) \tag{21.49}
$$

To determine the hoop stress distribution in the plastic zone, σ_{θ_p} , it must be remembered that

$$
\sigma_{\nu\rho} = \sigma_{\theta} - \sigma_{\rho}
$$

therefore

$$
\sigma_{\theta P} = \sigma_{\gamma P} \left\{ 1 + \ln \left(R_3 / R_1 \right) \right\} \tag{21.50}
$$

Plots of the stress distributions in a partially plastic cylinder, under internal pressure, are shown in Figure 21.16.

Figure 21.16 Stress distribution plots.

Thick circular cylinders, discs and spheres

Problem 21.5 A circular cylinder of 0.2 m external diameter and of 0.1 m internal diameter is **shrunk** onto another circular cylinder of external diameter 0.1 m and of bore 0.05 m, where the dimensions are nominal. If the interference fit is such that when an internal pressure of 10 MPa is applied to the inner face of the inner cylinder, the inner face of the inner cylinder is on the point of yielding. What internal pressure will cause plastic penetration through half the thickness of the inner cylinder. It may be assumed that the Young's modulus and Poisson's ratio for both cylinders is the same, but that the outer cylinder is made of a higher grade steel which will not yield under these conditions. The yield stress of the inner cylinder may be assumed to be 160 MPa.

Solution

The Lamé line for the compound cylinder at the onset of yield is shown in Figure 21.17.

Figure 21.17 Lamé line for compound cylinder.

In Figure 21.17,

 σ_1 = hoop stress on inner surface of inner cylinder.

0, = hoop stress on outer surface of inner cylinder.

 σ_1 = hoop stress on inner surface of outer cylinder.

As yield occurs on the inner surface of the inner surface when an internal pressure of 50 MPa is applied,

$$
\sigma_1 - (-100) = 160
$$

$$
\therefore \sigma_1 = 60 \text{ MPa}
$$

Equating similar triangles in Figure 21.17, we get

536

$$
\frac{\sigma_1 + 100}{400 + 400} = \frac{100 - P_c}{400 - 100}
$$

$$
\frac{160 \times 300}{800} = 100 - P_c
$$

$$
\therefore P_c = 40 \text{ MPa}
$$
 (21.51)

Similarly from Figure 21.17

$$
\frac{\sigma_2 + 100}{400 + 100} = \frac{100 - P_c}{400 - 100}
$$
\n(21.52)\n
\n
$$
\sigma_2 = 0
$$

Also from Figure 21.17,

$$
\frac{\sigma_3}{100 + 25} = \frac{P_c}{100 - 25}
$$
\n
$$
\therefore \sigma_3 = \frac{400 \times 125}{75} = 66.7 \text{ MPa}
$$
\n(21.53)

Consider, now, plastic penetration of the inner cylinder to a diameter **0.075.** The Lam6 line in the elastic zones will be as shown in Figure 21.17. From Figure 21.18,

$$
\sigma_6 + P_3 = 160
$$

Figure 21.18 Lamé line in elastic zones.

therefore

$$
\therefore \sigma_6 = 160 - P_3 \tag{21.54}
$$

Similarly

$$
\frac{P_3 - P_2}{400 - 100} = \frac{\sigma_6 + P_3}{400 + 400} = \frac{160}{800}
$$
 (21.55)

$$
\therefore P_3 = 60 + P_2 \tag{21.56}
$$

Also from Figure 21.18

$$
\frac{\sigma_4}{100 + 25} = \frac{P_2}{100 - 25} \tag{21.57}
$$

or
$$
\sigma_4 = 1.667 P_2
$$
 (21.58)

Substituting equation (21.56) into equation **(21.58),** we get

 σ_4 = 1.667 $(P_3$ - 60)

or
$$
\sigma_4 = 1.667 P_3 - 100
$$

Also from equation (21.55)

$$
\frac{\sigma_{5} + P_{3}}{100 + 400} = \frac{P_{3} - P_{2}}{400 - 100} = \frac{160}{800}
$$
\n
$$
\therefore \sigma_{5} = 100 - P_{3}
$$
\n(21.59)

Now,

$$
w = \frac{r}{E} \left(\sigma_{\theta} - \nu \sigma_{r} \right)
$$

which will be the same for both cylinders at the common surface, i.e.,
\n
$$
\frac{1}{E} \{(\sigma_5 - \sigma_2) - \nu(P_2 - P_c)\} = \left\{\frac{1}{E} (\sigma_4 - \sigma_3) - \nu(P_2 - P_c)\right\}
$$

 \overline{or}

$$
\sigma_5 - \sigma_2 = \sigma_4 - \sigma_3
$$

Substituting equations (21.52), (21.53), (21.58) and (21.59) into the above, we get

100 - P_3 - 0 = 1.667 P_3 - 100 - 66.7 2.667 P_3 = 100 + 100 + 66.7

or

$$
P_3 = 100
$$

Consider now the yielded portion

$$
\sigma_r = \sigma_{yp} \ln r + c
$$

\n
$$
\sigma_{yp} = 160
$$

\nat $r = 0.0375 \text{ m}$,
\n
$$
\sigma_r = -P_3 = -100
$$

\nor $-100 = 160 \ln (0.0375) + C$
\n $C = -100 + 525.3$
\n $\therefore C = 425.3$
\nNow, at $r = 0.025 \text{ m}$,

-P = 160 In (0.025) + 425.3 $=$ $-590.2 + 425.3$ *P* = 164.9 MPa

which is the pressure to cause plastic penetration.

Problem 21.6 Determine the internal pressure that will cause complete plastic collapse of the compound cylinder given that the yield stress for the material of the outer cylinder is 700 MPa.

Solution

Now,

$$
P_p = \sigma_{yp} \ln\left(\frac{R_3}{R_1}\right)
$$

\n
$$
= \sigma_{yp2} \ln\left(\frac{R_3}{R_2}\right) + \sigma_{yp1} \ln\left(\frac{R_2}{R_1}\right)
$$

\n
$$
= 700 \ln\left(\frac{0.1}{0.05}\right) + 160 \ln\left(\frac{0.05}{0.0375}\right)
$$

\n
$$
= 485 + 46
$$

\n
$$
P_p = 531 \text{ MPa}
$$
 (21.60)

which is the plastic collapse pressure of **the compound cylinder.**

21.6 Thick spherical shells

Consider a thick hemispherical shell element of **radius** *r,* **under a compressive radial stress** *P,* **as** shown in Figure 21.19.

Figure 21.19 Thick hemispherical shell element.

Let w be the radial deflection at any radius r ,

so that

$$
hoop strain = w/r
$$

and

hoop strain =
$$
w/r
$$

radial strain = $\frac{dw}{dr}$

From three-dimensional stress-strain relationships,

$$
E \frac{w}{r} = \sigma - v\sigma + vP \qquad (21.61)
$$

and

$$
E \frac{dw}{dr} = -P - v\sigma - v\sigma \qquad (21.62)
$$

$$
= -P - 2v\sigma
$$

Now

$$
Ew = \sigma r - v\sigma r + vP r
$$

which, on differentiating with respect to r , gives

$$
E \frac{dw}{dr} = \sigma + r \frac{d\sigma}{dr} - \nu \sigma - \nu r \frac{d\sigma}{dr} + \nu P + \nu r \frac{dP}{dr}
$$

$$
= (1 - \nu) \left(\sigma - r \frac{d\sigma}{dr} \right) + \nu \left(P + r \frac{dP}{dr} \right)
$$
 (21.63)

Equating (2 *1.62)* **and** *(2* **1.63),**

(21.62) and (21.63),
\n
$$
-P - 2v\sigma = (1 - v)\left(\sigma - r\frac{d\sigma}{dr}\right) + v\left(P + r\frac{dP}{dr}\right)
$$

or

$$
(1 + v) (\sigma + P) + r (1 - v) \frac{d\sigma}{dr} + vr \frac{dP}{dr} = 0
$$
 (21.64)

Considering now the equilibrium of the hemispherical shell element,

$$
\sigma \times 2\pi r \times dr = P \times \pi r^2 - (P + dP) \times \pi \times (r + dr)^2 \qquad (21.65)
$$

Neglecting higher order terms, equation 21.65 becomes
\n
$$
\sigma + P = (-r/2) \frac{dP}{dr}
$$
\n(21.66)

Substituting equation (21.66) into equation (21.64),

$$
-(r/2) (dP/dr) (1 + v) + r (1 - v) (d\sigma/dr) + vr (dP/dr) = 0
$$

or

$$
\frac{d\sigma}{dr} - \frac{1}{2} \frac{dP}{dr} = 0 \tag{21.67}
$$

which on integrating becomes,

$$
\sigma - P/2 = A \tag{21.68}
$$

Substituting equation (21.68) into equation (21.66)

$$
3P/2 + A = (-r/2) (dP/dr)
$$

or

$$
-\frac{1}{r^2}\frac{d(P\times r^3)}{dr} = 2A
$$

or

$$
\frac{d(P \times r^3)}{dr} = -2Ar^2
$$

which on integrating becomes,

 $P \times r^3 = -2Ar^{3}/3 + B$

or

$$
P = -2A/3 + B/r^3 \tag{21.69}
$$

and
$$
\sigma = 2A/3 + B/(2r^3)
$$
 (21.70)

21.7 Rotating discs

These are of much importance in engineering components that rotate at high speeds. If the speed is high enough, such components can shatter when the centrifugal stresses become too large. The theory for thick circular cylinders can be extended to deal with problems in this category.

Consider a uniform thickness disc, of density ρ , rotating at a constant angular velocity ω .

From

$$
E \frac{dw}{dr} = \sigma_r - \nu \sigma_\theta \tag{21.71}
$$

and,

$$
E \frac{w}{r} = \sigma_{\theta} - v \sigma_{r}
$$
 (21.72)

or,

$$
Ew = \sigma_{\theta} \times r - \nu \sigma_r \times r \tag{21.73}
$$

Differentiating equation (21.73) with respect to *r*,

$$
E \frac{dw}{dr} = \sigma_{\theta} + r \frac{d\sigma_{\theta}}{dr} - v\sigma_{r} - v r \frac{d\sigma_{r}}{dr}
$$
 (21.74)

Equating (21.71) and (21.74),

$$
\left(\sigma_{\theta} - \sigma_{r}\right)(1 + \nu) + r \frac{d\sigma_{\theta}}{dr} - \nu r \frac{d\sigma_{r}}{dr} = 0 \qquad (21.75)
$$

Considering radial equilibrium of an element of the disc, as shown in Figure 21.20,

$$
2\sigma_{\theta} \times dr \times \sin\left(\frac{d\theta}{2}\right) + \sigma_{r} \times r \times d\theta
$$

$$
- (\sigma_{r} + d\sigma_{r}) (r + dr)d\theta = \rho \times \omega^{2} \times r^{2} \times dr \times d\theta
$$

Figure 21.20 Element of disc.

In the limit, this reduces to

it, this reduces to
\n
$$
\sigma_{\theta} - \sigma_{r} - r \frac{d\sigma_{r}}{dr} = \rho \omega^{2} r^{2}
$$
\n(21.76)

Substituting equation (21.76) into equation (21.75) ,

ng equation (21.76) into equation (21.75),
\n
$$
\left(r \frac{d\sigma_r}{dr} + \rho \omega^2 r^2\right) (1 + v) + r \frac{d\sigma_\theta}{dr} - v r \frac{d\sigma_r}{dr} = 0
$$

or,

$$
\frac{d\sigma_{\theta}}{dr} + \frac{d\sigma_{r}}{dr} = -\rho \omega^{2} r^{2} (1 + v)
$$

which on integrating becomes,

$$
\sigma_{\theta} + \sigma_{r} = -(\rho \omega^{2} r^{2}/2) (1 + v) + 2A \qquad (21.77)
$$

Subtracting equation (21.76) from equation (21.77),

g equation (21.76) from equation (21.77),
\n
$$
2\sigma_r + r \frac{d\sigma_r}{dr} = -(\rho \omega^2 r^2/2) (3 + v) + 2A
$$

or,

$$
\frac{1}{r} \frac{d(\sigma_r \times r^2)}{dr} = - \frac{\rho \omega^2 r^2 (3 + v)}{2} + 2A
$$

which on integrating becomes,

$$
\sigma_r \ r^2 = -(\rho \omega^2 r^4 / 8)(3 + v) + Ar^2 - B \qquad (21.78)
$$

or

$$
\sigma_r = A - B/r^2 - (3+v)(\rho \omega^2 r^2 / 8)
$$

and,

$$
\sigma_{\theta} = A + B / r^2 - (1 + 3\nu) (\rho \omega^2 r^2 / 8)
$$
 (21.79)

Problem 21.7 Obtain an expression for the variation in the thickness of a disc, in its radial direction, so that it will be of constant strength when it is rotated at an angular velocity **w.**

Solution

Let,

 t_0 = thickness at centre $t =$ thickness at a radius $r =$ $t + dt$ = thickness at a radius $r + dr$ σ = stress = constant (everywhere)

Consider the radial equilibrium of an element of this disc at any radius r as shown in Figure 21.21.

Figure 21.21 Element of constant strength disc.

Resolving forces radially

$$
2\sigma \times t \times dr \sin\left(\frac{d\theta}{2}\right) + \sigma tr d\theta = \sigma(r + dr) (t + dt) d\theta + \rho \omega^2 r^2 t d\theta dr
$$

Neglecting higher order terms, this equation becomes

$$
\sigma t \, dt = \sigma r \, dt + \sigma t \, dr + \rho \omega^2 r t \, dr
$$

or

$$
\frac{\mathrm{d}t}{\mathrm{d}r} = -\rho \omega^2 r t/\sigma
$$

which on integrating becomes,

$$
\ln t = -\rho \omega^2 r^2 t/(2\sigma) + \ln C
$$

 $\overline{\text{or}}$

$$
t = Ce^{(-\rho \omega^2 r^2/2\sigma)}
$$

Now, at
$$
r = 0
$$
, $t = t_0$: $C = t_0$

Hence,

$$
t = t_0 e^{(-\rho \omega^2 r^2 / 2\sigma)} \tag{21.80}
$$

21.7.1 Plastic collapse of rotating discs

Assume that $\sigma_{\theta} > \sigma_{r}$, and that plastic collapse occurs when

$$
\sigma_{\theta} = \sigma_{yp}
$$

where σ_{yp} is the yield stress.

Let *R* be the external radius of the disc. Then,

from *equilibrium considerations,*

librium considerations,
\n
$$
\sigma_{yp} - \sigma_r - r \frac{d\sigma_r}{dr} = \rho \omega^2 r^2
$$

or,

$$
\int r d\sigma_r = \int \{ \sigma_{yp} - \sigma_r - \rho \omega^2 r^2 \} dr
$$

Integrating the left-hand side of the above equation by parts,

$$
r \sigma_r - \int \sigma_r dr = \sigma_{yp} r - \int \sigma_r dr \rho \omega^2 r^3/3 + A
$$

therefore

$$
\sigma_r = \sigma_{yp} - \rho \omega^2 r^2 / 3 + A/r \qquad (21.81)
$$

For a solid disc, at $r = 0$, $\sigma_r \neq \infty$, or the disc will collapse at small values of ω . Therefore

$$
A = 0
$$

and

$$
\sigma_r = \sigma_{yp} - \rho \omega^2 r^2 / 3
$$

at $r = R$, $\sigma_r = 0$; therefore

$$
0 = \sigma_{vp} - \rho \omega^2 R^2 / 3
$$

$$
\therefore \omega = \frac{1}{R} \sqrt{\frac{3\sigma_{yp}}{\rho}}
$$
 (21.82)

where, ω is the angular velocity of the disc, which causes plastic collapse of the disc.

for equation (21.81) are: For an *annular disc*, of internal radius R_1 and external radius R_2 , suitable boundary conditions

at $r = R_2$, $\sigma_r = 0$; therefore

$$
A = \left(\rho \omega^2 R_1^2 / 3 - \sigma_{\text{yp}} \right) R_1
$$

$$
\therefore \sigma_r = \sigma_{yp} - \rho \omega^2 r^2 / 3 + \left(\rho \omega^2 R_1^2 / 3 - \sigma_{yp} \right) \left(R_1 / r \right) \tag{21.83}
$$

at $r = R_2$, $\sigma_r = 0$; therefore

$$
0 = \sigma_{yp} - \rho \omega^2 R_2^2 / 3 + \left(\rho \omega^2 R_1^2 / 3 - \sigma_{yp} \right) \left(R_1 / R_2 \right)
$$

Hence, $\omega = \sqrt{\left\{ \frac{3\sigma_{yp}}{\rho} \right\} \frac{\left(R_2 - R_1 \right)}{\left(R_2^3 - R_1^3 \right)}}$ (21.84)

21.8 Collapse *of* **rotating rings**

Consider the radial equilibrium of the thin semicircular ring element shown in Figure 21.21.

Figure 21.21 Ring element.

Let,

- $a = \text{cross-sectional area of ring}$
- R = mean radius of ring

Resolving forces vertically

$$
\sigma_{\theta} \times a \times 2 = \int_0^{\pi} \rho \omega^2 R^2 a \, d\theta \sin\theta
$$

$$
= \rho \omega^2 R^2 a \left[-\cos\theta \right]_0^{\pi}
$$

$$
= 2\rho \omega^2 R^2 a
$$

$$
\therefore \sigma_{\theta} = \rho \omega^2 R^2
$$

at collapse,

$$
\sigma_{\theta} = \sigma_{yp}
$$

\n
$$
\therefore \omega = \frac{1}{R} \sqrt{\left(\frac{\sigma_{yp}}{\rho}\right)}
$$
\n(21.85)

where *o* **is the angular velocity required to fracture the** ring.